



Aeration length phase control using on-line ammonia and dissolved oxygen feedback control at Choutrana II WWTP

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ABSTRACT

The purpose of this work was to study the impact of aeration control strategy using on-line ammonia and dissolved oxygen feedback control on energy consumption in a full-scale wastewater treatment plant. Eight identical oxidation ditches treating the same effluent and operated with conventional control aeration strategy with distinct nitrification and denitrification phases were examined. Each oxidation ditch was periodically aerated according to a fixed phase cycle and phase lengths are controlled by constant dissolved oxygen (DO) set points. To improve the plant's energy efficiency, a new control strategy has been performed by a full-scale test to control the phase lengths by variable DO set points determined by criteria functions. Knowing the phase lengths provides flexibility to achieve easily nitrogen treatment with nitrification and denitrification processes and treatment objectives. In the test strategy, each zone was separately controlled and the DO set points were changed to avoid unnecessarily high DO levels. The results showed that the ammonium and DO feedback control method can reduce about 30% of the total airflow and lead to a significant aeration energy savings. The plant could preserve treatment efficiency, enhance simultaneous nitrification-denitrification process, and produce excellent effluent quality. A Ammonia and DO feedback controller system was implemented and compared with the actual DO control system.

Keywords: Aeration control strategy; Ammonium; DO feedback; Energy consumption; DO set points

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1. Introduction

The assessment of energy consumption in the activated sludge plants revealed that the aeration of the biological treatment process can reach up to 70% of the electrical consumption at the plant [1]. DO is used for the removal of ammonium through the process of nitrification. Hence, it makes good sense to relate the removal of ammonium to the energy consumption in plants where denitrification is included (a large part of the biodegradable organic matter is removed in the denitrification process). The best way to do this is to relate the removal only to the aeration energy consumption. Very low effluent ammonium concentrations compared to the discharge limits lead to an inefficient aeration and cause an unnecessary costs in many plants [2–4]. To achieve nitrogen removal, a control process and a DO management are necessary by using on-line supervisory control. So, energy can be saved without deteriorating effluent quality. By the simultaneous nitrification–denitrification (SND) process, nitrogen removal is the key to appropriately set the aeration system and to establish sufficiently aerobic and anoxic zones simultaneously [5]. Several sophisticated control systems based on monitoring both nitrate and ammonia were used in full-scale implementations [6–8]. SND is a well-known phenomenon in biological nutrient removal in activated sludge process. SND has been reported more frequently in oxidation ditches, which operate with low DO concentrations in the majority of their volume. Without controlling SND mechanisms, the reduction of the air flow can decrease DO concentrations in aerated zones where nitrifying organisms take time to develop their population niches, as well as to acclimate to the low DO environment. These conditions may inhibit nitrification process, cause the development of filamentous bacteria, and destroy effluent quality by raising ammonia and nitrite concentrations [9]. When alternating aerobic and anoxic conditions in a full-scale plant where SND has been achieved, nitrified biomasses need to be established. It has been found that SND can reduce carbon, oxygen, and alkalinity consumption by 10–40%, depending on the degree of SND achieved [10].

Different nitrogen removal methods are presented in Samuelsson [11]. Several of those are related to denitrification and the addition of external carbon. The work related to aeration control includes the control of the aerobic volume. Lumley [12] found that return and waste sludge rates are the two principal variables to adjust nitrate concentration and sludge age, respectively. Holenda [13] investigated methods to optimize the aeration length in an alternating system, but also

developed a model predictive control (MPC) for DO set point tracking. The MPC is based on a linearized version of the ASM1.

Many researchers improved that dynamic adaptation of the aerated volume to alternate loading conditions is the key to maximize the plant performance and to reduce the energy consumption. It is in this context that we will conduct a study based on the simulation using ASM1 model for testing online ammonia monitoring and DO feedback controllers (Fig. 3) in order to:

- (1) Define anoxic/oxic lengths, respectively, using on-line ammonia and DO feedback control in full-scale plant in Tunisia.
- (2) Develop a control strategy via online nutrient and DO feedback for dynamic adaptation to load variations to maximize the plant efficiency;
- (3) Show the role that online nutrient instrumentations have played at the plant in terms of efficiency, energy, and chemical savings.
- (4) Use these instrumentations in our existing and future WWTPs.

A successful application of the new strategy in Choutrana II plant could provide an opportunity for future process control automation based on continuous online measurement of DO and nutrient parameters.

2. Methods and materials

2.1. Choutrana II WWTP

Choutrana II plant operates continuously throughout the year and treats 40,000 m³/d of sewage, 40,000 kg/d of chemical oxygen demand (COD), and 3,000 kg/d of nitrogen corresponding to the equivalent of about 600,000 inhabitants served. It is composed of four parallel lines. Each line consists of an oxidation ditch. It treats about 20,000 kg BOD₅/d with low load of 0.08 gBOD/gTSS/d. Hydraulic retention time is about 22 h and sludge residence time is 18.5 d. DO is served by a fine bubble diffused air system. The return activated sludge (RAS) flow rate is about 75% of the influent flow.

Note that from the standard practice, the RAS flow rate is considered as 150% of the influent flow during dry weather and 100% of the flow during wet weather.

Fig. 1 presents a schematic flow diagram of the liquid process at the plant. Each oxidation ditch is

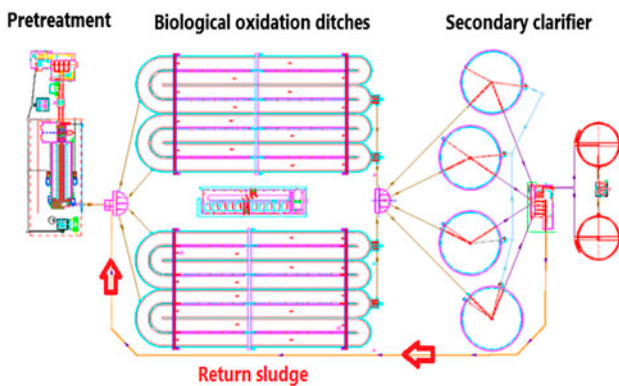


Fig. 1. Schematic flow diagram of Choutrana II plant.

equipped with diffusers and four agitators. In addition, each oxidation ditch is equipped with DO probes which are used in the automatic DO control.

2.2. Aeration in oxidation ditch

Fig. 2 shows aerated and non-aerated zones in oxidation ditch. The first anoxic zone receives raw wastewater. The air diffusers are placed on the bottom of the tanks. The airflow rate through the diffuser is 4.36 Nm³/h (but it is variable between 3.5 and 6.5 Nm³/h). The number of diffusers per zone increases and the highest number is located in zone 6. However, the COD and ammonium loads decrease from zone 1 to zone 9 when the flow is passed through multiple anoxic/aerobic zones.

Table 1 shows the number of diffusers, the airflow in each aerated zone, and the volume of each zone.

2.3. DO control system

Since the start-up in 2007, Choutrana II WWTP has been operated firstly with a constant DO set point (2 mgO₂/L) in oxidation ditch where two DO probes were installed to control the airflow (Fig. 2). Currently,

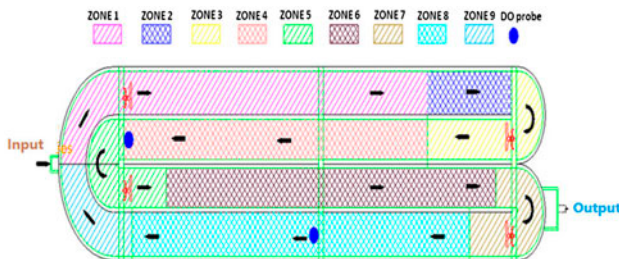


Fig. 2. Aerated and anoxic zones in oxidation ditch. Locations of DO probe ●.

aeration control relies on the operator experience and manual mode. The function of blowers is controlled by a timer with fixed phase length: 45 min of aeration and 15 min of non-aeration. The blowers work 18 h/d. Although the plant was equipped with DO probes, aeration adjustment has not been fully completed.

Borglund [14] tested a strategy during low and high loading conditions to control air flow in full scale. It is based on a linear air flow distribution designed to gradually decrease the air flow to the aerated zones according to a step-like structure. Two DO probes are used to control the aeration system. The first probe situated in the first zone controls the total airflow to the oxidation ditch. The second one situated in the last zone controls the inclination of the step-like structure. When the DO concentration in the last zone deviates from its DO set point, the inclination is shifted, affecting the DO concentration in the first zone. This causes the DO probe in the first zone to change the total airflow to the oxidation ditch. The strategy during low and high loading conditions is illustrated in Fig. 3. There are four zones in the illustration, but the principle is the same also in tanks with several zones.

The original DO control strategy at Choutrana II plant (Fig. 2) is based on constant and variable airflow distributions in aerated zones. Table 1 shows airflow distribution in each aerated zone. The amount of oxygen transferred per added volume of air is different in aerated zones. This is not advantageous from the point of view of energy.

2.4. Models

2.4.1. Oxygen transfer model

According to Mulas [15], a change in airflow affects the DO concentration. Eq. (1) describes this relation:

$$\frac{dC(t)}{dt} = K_{La}(q_{air}(t))(C_{sat} - C(t)) - \frac{Q}{V}(C_{in}(t) - C(t)) - R(t) \tag{1}$$

where $K_{La}(q_{air})$ = oxygen transfer rate as a function of airflow (l/h), C_{sat} = saturated oxygen level (mg/L), $C(t)$ = oxygen concentration (mg/L), $C_{in}(t)$ = oxygen concentration of incoming water (mg/L), $R(t)$ = oxygen consumption rate due to microbial activity (mg/L h), $Q(t)$ = flow (m³/h), V = aerated volume (m³).

Lindberg [16] showed that a nonlinear relationship between the airflow rate and the oxygen transfer coefficient (K_{La}) can be considered.

Table 1
Aeration system in oxidation ditch

Zone	Air flow/zone (Nm ³ /h)	Number of diffusers	Volume of zone (m ³)	Length of zone (m)
1	0	0	2,408	80
2	1,500	344	602	20
3	0	0	1,204	40
4	8,000	1835	2,408	80
5	0	0	1,204	40
6	5,000	1,146	3,010	100
7	0	0	602	20
8	2,500	573	3,010	100
9	0	0	595	20
Total	17,000	3,898	15,000	500

Note: Bold values define the oxidation ditch design and is the sum of each value range.

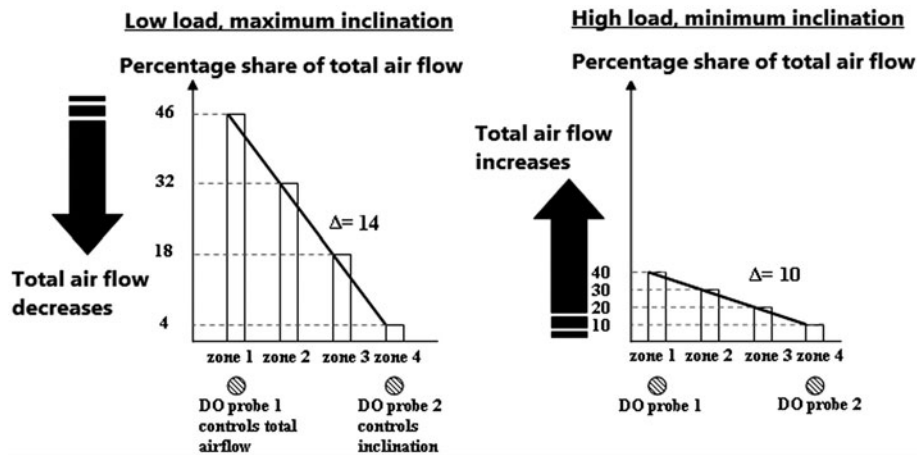


Fig. 3. Air flow profile during low and high load [14].

$$K_{La}(q_{air}) = K_1(1 - e^{-k_2q_{air}}) \quad (2)$$

where q_{air} = airflow rate (m³/h), $k_1 = (1/1 - K_{La}\tau)(C_0 - C_{Sat})$, $k_2 = (K_{La}\tau/K_{La}\tau - 1)(C_0 - C_{Sat})$, τ = delay times of the oximeter (s), C_0 = Initial oxygen level (mg/L).

Capela et al. [17] showed, in specific conditions, that the oxygen transfer coefficient (K_{La}), increases linearly with the aeration rate and the relationship between the K_{La} function and airflow rate can be expected to be nonlinear.

From Eq. (1), it can be seen that the driving force for mass transfer of oxygen is lower at DO concentrations close to the level of oxygen saturation. Since high airflows lead to high DO concentrations, and because of the nonlinear structure of the K_{La} -function, high airflows give an inefficient oxygen transfer.

2.4.2. Blowers energy consumption

Ingildsen [18] carried out an experiment and found that the airflow rate could be translated into power by the Eq. (3):

$$\text{Power} = 0.0115 \text{ kW}/(\text{h}/\text{Nm}^3) \times \text{Airflow} + n \times 28 \text{ kW} \quad (3)$$

where n is the number of blowers in operation. This means that having a blowers running without yielding any air requires 28 kW.

2.4.3. Simulation model

The simulation software used was GPS-X (Hydromantis Inc.). The ASM1 model was selected for the simulation. For the aeration tank, new control

mode will be tested and will be compared to the current one.

- (a) On-Off control defined by cycle times (current mode)
- (b) On-Off function times of the blowers (aeration length phase) using online ammonia concentration and DO feedback: Times of the blowers recorded at the full-scale plant will be applied directly to control data input for dynamic simulations on real data.

2.4.4. Kinetic of nitrifying biomass

Biomass growth and decay rates are the key parameters for a successful biological treatment [19]. According to Svardal et al. [20], the growth rate of nitrifying biomasses depends on several factors where DO and ammonium concentrations are the most important parameters. The nitrifying bacteria can reach 80% of their maximum growth at an ammonia concentration of 2 mg/L.

Svardal et al. [20] and Regmi et al. [27] showed that the nitrification rate at low ammonium concentration (<1–2 mg NH₄-N/L) is linearly dependent on the DO concentration. The gradient of the growth rate decreases as the concentration increases. Due to this behavior, there is a potential to decrease high DO concentrations in an aerated zone with only a minor decrease of the growth rate of the nitrifying bacteria. Similarly, there is a potential to increase the growth rate substantially by increasing the ammonium concentration in an aerated zone where it is low.

The growth rate is commonly expressed according to Monod kinetics [21] and it is given by the following equation:

$$\mu = \mu_{\max} \left(\frac{S}{K_s + S} \right) \quad (4)$$

where μ : growth rate (d⁻¹), μ_{\max} : maximum growth rate (d⁻¹), S : Substrate concentration (g m⁻³), K_s : Half saturation coefficient (gCOD m⁻³).

Eqs. (5), (6), and (7) show the relation between the nitrification rates in aerated and anoxic phases.

2.4.4.1. Growth rate of aerobic autotrophs.

$$r = \mu_{A\max} \left(\frac{S_{\text{NH}}}{K_{\text{NH}} + S_{\text{NH}}} \right) \left(\frac{S_{\text{O}}}{K_{\text{OA}} + S_{\text{O}}} \right) X_{\text{BA}} \quad (5)$$

where $\mu_{A\max}$ = maximum growth rate of autotrophic bacteria (d⁻¹), S_{NH} = ammonium concentration (gNH₄⁺ m⁻³), K_{NH} = ammonia half-saturation coefficient for autotrophs (gN m⁻³), K_{OA} = oxygen half saturation constant for autotrophs (g m⁻³), X_{BA} = autotrophic biomass (gCOD m⁻³).

2.4.4.2. Growth rate of aerobic heterotrophic biomass.

$$r = \mu_{\text{Hmax}} \left(\frac{S_{\text{S}}}{K_{\text{S}} + S_{\text{S}}} \right) \left(\frac{S_{\text{O}}}{K_{\text{OH}} + S_{\text{O}}} \right) X_{\text{BH}} \quad (6)$$

where μ_{Hmax} = maximum growth rate of heterotrophic biomass (d⁻¹), S_{S} = COD concentration rapidly biodegradable (gCOD m⁻³), K_{S} = half-saturation coefficient of heterotrophic biomass (gCOD m⁻³), X_{BH} = heterotrophic biomass (gCOD m⁻³).

2.4.4.3. Growth rate of anoxic heterotrophic biomass.

$$r = \eta_{\text{NO}} \frac{S_{\text{NO}}}{K_{\text{NO}} + S_{\text{NO}}} \mu_{\text{Hmax}} \left(\frac{S_{\text{S}}}{K_{\text{S}} + S_{\text{S}}} \right) \left(\frac{S_{\text{O}}}{K_{\text{OH}} + S_{\text{O}}} \right) X_{\text{BH}} \quad (7)$$

where η_{NO} = correction factor for μ_{H} under anoxic conditions, K_{NO} = nitrate half-saturation coefficient for denitrifying biomass (gN m⁻³), S_{NO} = nitrates + nitrites (gN m⁻³).

The Eq. (8) shows that the nitrification rate r_{N} depends on the ammonia concentration S_{NH} and subsequently r_{N} is directly proportional to the oxygen uptake rate OUR_{N} (Eq. (8)).

$$\text{OUR}_{\text{N}} = (4.57 - Y_{\text{A}}) \times r_{\text{N}} \quad (8)$$

where OUR_{N} : Oxygen Uptake Rate, Y_{A} : yield for autotrophic biomass (gCOD gN⁻¹).

Eq. (9) indicates that the total oxygen requirement is a function of air flow in the reactor. A linear relationship between DO concentration and nitrification rate provides useful information for process control:

$$\text{TOR} = \text{OUR}_{\text{N}} \times V \times \frac{C_{\text{S}}}{C_{\text{S}} + S_{\text{O}}} \quad (9)$$

where TOR: Total oxygen requirement, V : Airflow volume (Nm³/h), C_{S} : Oxygen level (mg/L), S_{O} : Substrate (mg/L).

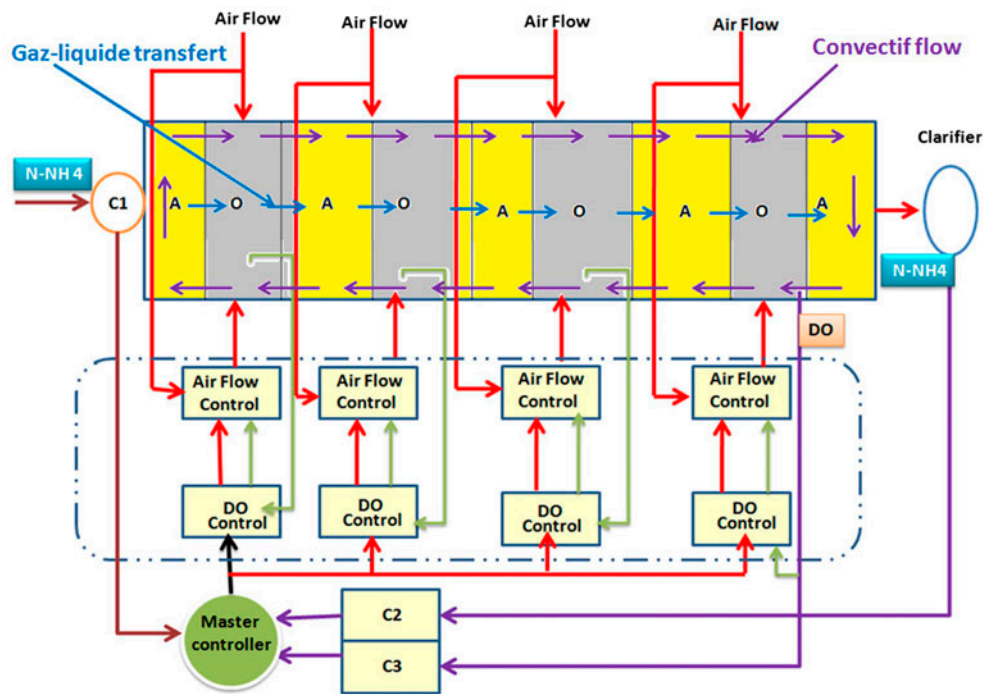


Fig. 5. DO cascade control.

the second controller regulates the ammonia level by handling the airflow rate (Q_{air} in $\text{Nm}^3 \text{d}^{-1}$).

The controllers (C1, C2, C3, and master controller) used are Predictive Integral Controllers (PICs). PICs are an implementation of Model Predictive Controllers originally proposed by Regmi et al. [27]. They are based on an internal model of the process that ensure fast disturbance rejection and robustness around a nominal point of functioning. In our case, controller's behavior can be expressed with first-order linear models as a relation between nitrate and ammonia and between ammonia and oxygen transfer coefficient. Eqs. (10) and (11) show an example of criteria functions [28–30]:

$$\text{NO}_{\text{MIN}} = \alpha \text{NH}_4\text{-N} + \beta \quad (10)$$

$$\text{NH}_{\text{MIN}} = \gamma \text{NO}_3\text{-N} + \delta \quad (11)$$

where α , β , γ , and δ are predefined parameters.

$\text{NH}_4\text{-N}$ and $\text{NO}_3\text{-N}$ are the measured ammonia and nitrate concentrations, respectively

This includes a considerable simplification, but it ensures a simple parameterization of the control model and still provides a very good performance [5]. Even with these simple models, the parameterization on real processes is not easy as the inlet pollution is

continuously disturbing these models and the gain and time constants of each transfer functions are not easy to identify. The parameters that can be optimized in this control law are:

- (1) the set point of nitrate or DO and
- (2) the upper and lower limits of ammonia.

These two limit parameters are necessary to avoid high and low ammonia levels. They also will allow that the nitrate level will be reduced during the night when ammonia levels are low and it will increase during the peak of ammonia.

The fundamental principle is ending nitrification phases when ammonia is "low" and is stopping denitrification phases when nitrate is "low" [31,32]. Several control strategies have been reported to implement this principle.

Three different scenarios are tested at Choutrana II plant. Table 3 summarizes the tests carried out.

3. Results and discussion

The strategy has been implemented at one line of Choutrana II WWTP during six days, from March 24 to 29, 2014. During the experiment, various simulations were done. The aim of this study was to reach a concordance between simulated and measured DO

Table 3
Summary of tests

Test	NH ₄ length ^a (m)	NO ₃ length ^b (m)	Air flow in aerated zones	Anoxic fraction ^c (%)	Oxic fraction ^d (%)
Reference (actual strategy)	170	330	Z2 = 1,500 Nm ³ /h Z4 = 8,000 Nm ³ /h Z6 = 5,000 Nm ³ /h Z8 = 2,500 Nm ³ /h	34	66
Test 1	190	310	Z2 = 0 Nm ³ /h + (Z4 = 5,000 Nm ³ /h)	38	62
Test 2	200	300	Test 1 + (Z6 = 3,500 Nm ³ /h)	45	55
Test 3	250	250	Test 1 + (Z8 = 1,500 Nm ³ /h)	50	50

^adevelopment length of anoxic zones.

^bdevelopment length of oxic zones.

^cAnoxic fraction = NH₄ length/(NH₄ length + NO₃ length).

^dOxic fraction = NO₃ length/(NH₄ length + NO₃ length).

in biological line, and save at the same time the aeration energy in terms of total airflow rates to the biological line. One of the experimental tests (test 2) aimed to enhance SND in oxidation ditch according to the inlet load. This test was implemented by using gain functions which yielded lower airflow and DO levels in the three last zones (zones 7, 8, and 9).

Fig. 6 shows that the strategy gave a reduction of the total airflow of 30% (from 15,000 to 10,500 Nm³/h). This reduction was mainly due to the better use of the aerated volumes. The zone 2 can be switched-on in peak loads. The control model adapted the DO set points to the influent load variations giving additional reductions. Especially when the load decreases at night and when no personnel is at the plant during weekends, large savings will be made.

With the proposed strategy, DO levels are controlled and the first three zones are used for denitrification process. The aerated zone 4 uses the high airflow and the maximum DO set points (from Fig. 7a, simulated level is 3.12 mg/L and measured level is 3.68 mg/L). This situation decreases the air-

flows in the following zones because the majority of ammonium and organic carbon is removed in the first aerobic zones. The remaining loads are pushed to the following zones which are effectively aerated depending on these loads.

Figs. 7a and 8a show the evolution of DO concentrations in experimental and current line in oxidation ditch. In the experimental line, the DO concentrations range between 0.1 and 3.02 mg/L in anoxic and oxic zones, respectively (Fig. 7a), however the current management leads to high DO levels which can inhibit the denitrification process in anoxic zones. Fig. 8a shows that DO concentrations in zones 4–9 range between 2.8 and 4.4 mg/L where denitrification process cannot take place and the nitrate effluent concentration is very high, about 60 mg/L, while in zones 1, 2, and 3, return nitrate could be removed by denitrification process (Fig. 8b). This is a highly inefficient use of the aired volumes resulting in unnecessarily high energy consumption. Fig. 7a shows the DO variation in oxidation ditch. The DO concentration decreases in the first, the second and the third anoxic zones where the return nitrate is reduced to nitrogen gaseous and increases in the following zones to oxidize the ammonia to nitrites and nitrates. The maximum DO set point, founded in the fourth zone (about 3 mg/L), could be used as a reference to operate the aeration system until the installation of the nitrogen probes.

Table 4 shows the treatment efficiency during evaluation of the strategy. It is shown that this strategy could contribute to significant improvements in the management of the plant and could be valuable to optimize the treatment efficiency and the energy savings.

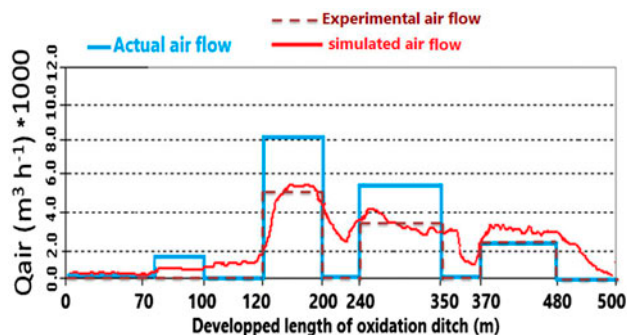


Fig. 6. Total airflows in the various zones.

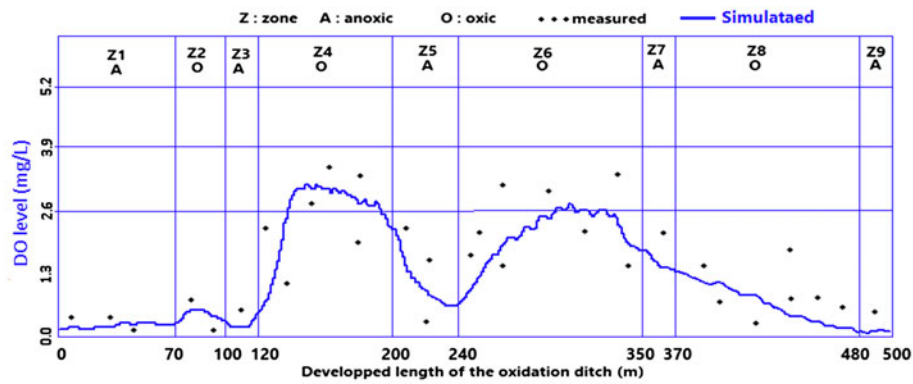


Fig. 7a. DO levels in the various zones.

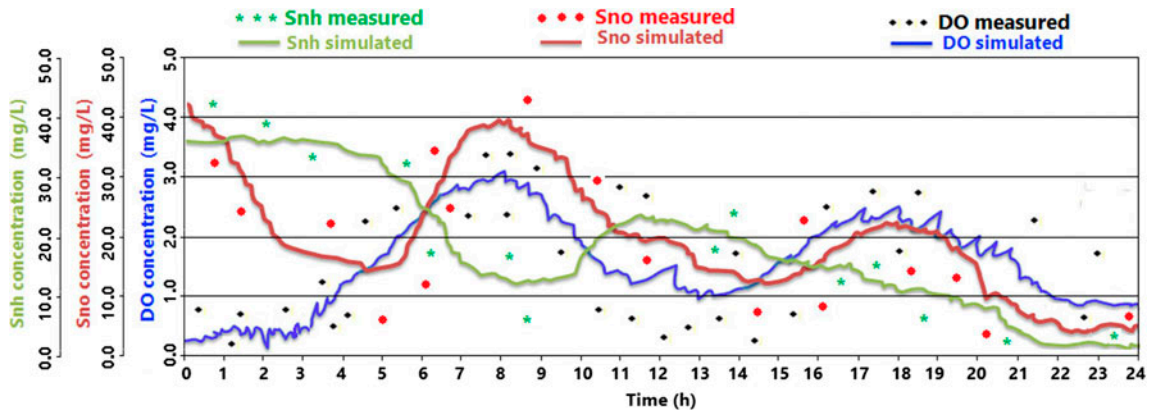


Fig. 7b. Nitrogen removal in the various zones. Notes: Snh: Ammonia concentration in mg/L and Sno: Nitrate and nitrite concentration in mg/L.

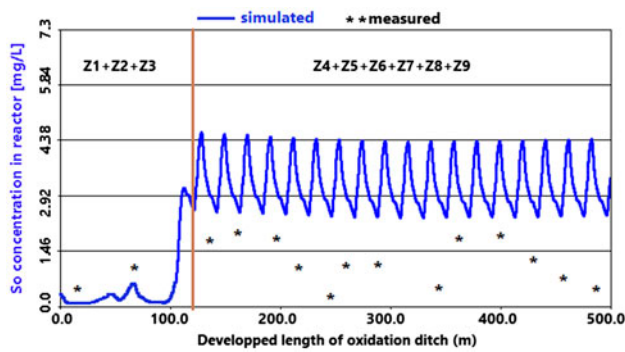


Fig. 8a. DO levels in the various zones. Notes: Snh: Ammonia concentration in mg/L and Sno: Nitrate and nitrite concentration in mg/L.

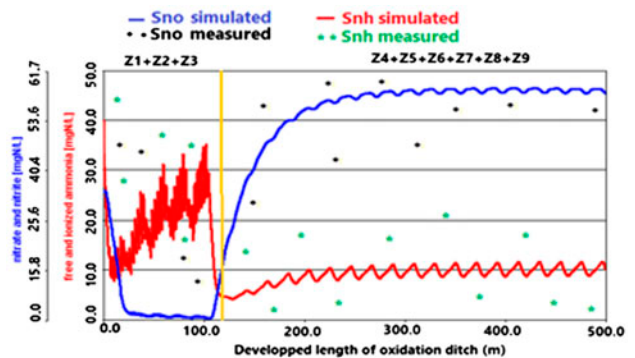


Fig. 8b. Nitrogen removal in the various zones.

The filamentous bacteria could be developed in the case of a significant reduction in DO with a high sludge age (about 20 d) [15]. Sludge volume index (SVI) tests have been performed two times at the end

of the experiment and did not indicate the presence of such problems. The SVI values were 98 ml/g in the first test and 105 ml/g in second one. These values are similar to those measured by the operator. The SVIs

Table 4
Treatment efficiency with the new strategy

Date	COD (mg/L)			N-NH ₄ (mg/L)			TKN (mg/L)		
	Input	Output	Treatment efficiency (%)	Input	Output	Treatment efficiency (%)	Input	Output	Treatment efficiency (%)
24 March 2014	1,352	85	94	44	5	84	58	11	81
25 March 2014	1,085	92	92	38	10	79	45	8	82
26 March 2014	938	88	91	48	10	88	55	10	82
27 March 2014	1,120	87	92	44	8	82	51	12	76
28 March 2014	987	85	91	37	6	84	48	9	81
29 March 2014	1,050	80	92	45	5	89	53	13	75

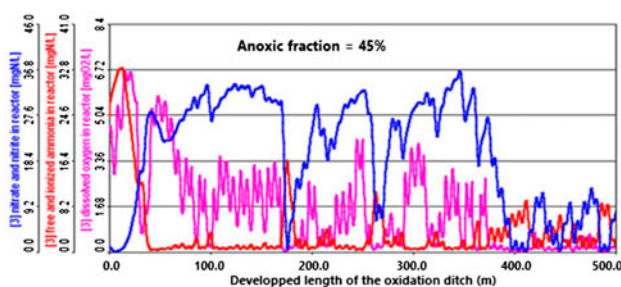


Fig. 9. Predicted ammonia, nitrate, and DO profiles in ditch.

are in a normal range. This result indicates that the extension of anoxic conditions did not reach the threshold which causes the appearance of filamentous bacteria. Microscopic investigations of the sludge have not been performed.

3.1. Strategy performance

Predictions of the effluent ammonium concentration based on controller airflow with ammonium and DO feedback as illustrated by Fig. 9 have shown to

be rather efficient to the plant. These predictions are based on the nitrification rate (Eq. (5)) which describes the dependency on DO set points and by estimating the parameters r_{nit} , K_{NH} , and K_{OA} with simulation by ASM1 model. The optimization is based on the minimization of the difference between measured and simulated effluent ammonium concentrations.

The new strategy was tested on 24 d. The simulation results with ASM1 model can be seen in Figs. 9 and 10. Fig. 9 shows the simulation of the evolution of the ammonium, nitrate, and the DO in the ditch. By using the ASM1 model, we can decrease the oxidic fraction from 66 to 55%, and increase the anoxic fraction from 34 to 45% (Table 3) thus saving energy.

Fig. 10 shows the fluctuation of the ammonium concentration between 0.2 and 11 mg N-NH₄/L. This variation could be explained by the fact that several processes influencing ammonium are not considered especially the ammonification, heterotrophic growth, and hydrodynamic in the oxidation ditch (a completely mixed reactor is assumed). The variation of the key parameters can be seen in Fig. 11, maximum removal rate (r_{max}) can be seen in Fig. 11(a), and the evolution of K_{OA} and K_{NH} can be seen in Fig. 11(b).

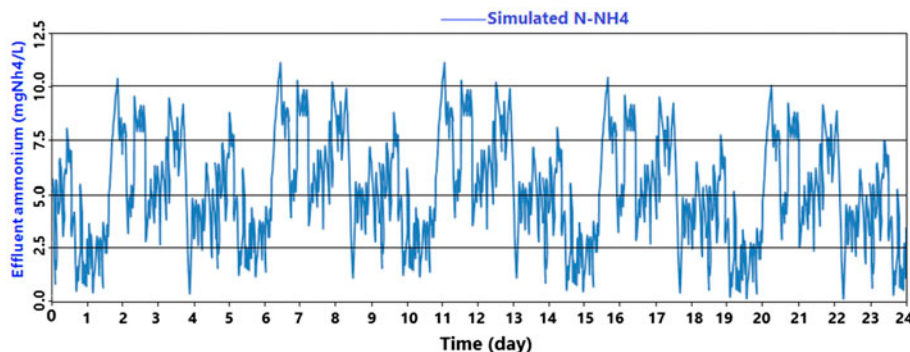


Fig. 10. Predicted of the plant performance based on the strategy controller airflow with ammonium and DO FFFB.

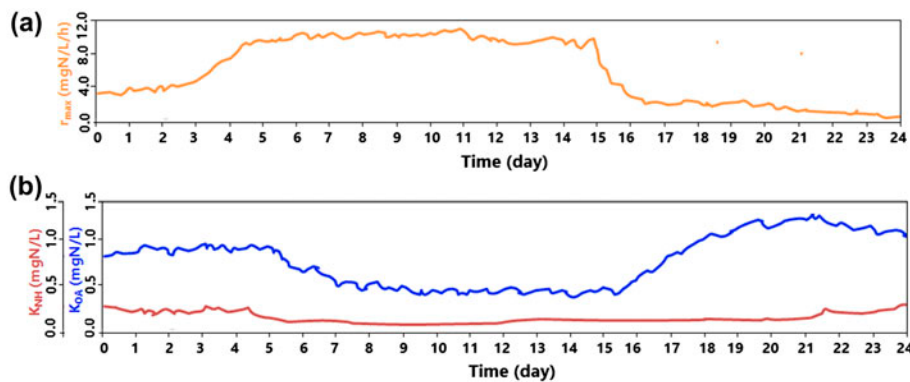


Fig. 11. Evolution of r_{nit} (a); K_{OA} and K_{NH} (b) based on the ASM1 model.

The parameters show relatively small variation, which indicate that the strategy is practically stable over the experimental period. The ASM 1 will be used to perform a simulation taking into account others parameters (ammonification, heterotrophic growth, etc.).

3.2. Savings

Since the strategy was tested in one line, it induces a reduction of the airflow of about 30% (4,500 Nm³/h). The savings become substantial about 18,000 Nm³/h when the strategy will be implemented to all lines of the plant. A reduction of the airflows of 30% corresponds to a yearly savings of approximately 125,000 \$. Instrumentation costs are close about 38,000 \$/year and maintenance costs are about 7,500 \$/year.

4. Conclusions

A strategy controller of the airflow with ammonium and DO feedback control with online measurements of the ammonium load has been proposed and tested in a full-scale plant over 6 d. Preliminary results show that the suggested strategy can lead to a decrease in the DO concentration during large parts of the day. It can save 30% of the airflow and improve effluent quality.

Simulation results over 24 d indicate that the strategy is practically stable and can be implemented to the plant.

This work shows that the setup of online instrumentations is critical for real time response to the load changes. The benefit of online instrumentation is substantial compared to intensive sampling and analysis.

The results of this experiment could be an opportunity to install nitrogen sensors for online measurements of the ammonia and DO concentrations in the existing and future Tunisian WWTP.

References

- [1] Y. Fayolle, S. Gillot, A. Cockx, L. Bensimhon, M. Roustan, A. Heduit, In situ characterization of local hydrodynamic parameters in closed-loop aeration tanks, *Chem. Eng. J.* 158 (2010) 207–212.
- [2] M. Ekman, B. Björnlénus, M. Andersson, Control of the aeration volume in an activated sludge process using supervisory control strategies, *Water Res.* 40 (2006) 1668–1676.
- [3] C. Sahlmann, J.A. Libra, A. Schuchardt, U. Wiesmann, R. Gnirrs, A control strategy for reducing aeration costs during low loading periods, *Water Sci. Technol.* 50(7) (2004) 61–68.
- [4] D. Vrecko, N. Hvala, A. Stare, M. Strazar, M. Levstek, P. Cerar, S. Podbevsek, Improvement of ammonia removal in activated sludge process with feedforward-feedback aeration controllers, *Water Sci. Technol.* 53 (4–5) (2006) 125–132.
- [5] H.W. Zhao, A.J. Freed, R.W. Dimassimo, S.N. Hong, E. Bundgaard, H.A. Thomsen Kruger, Demonstration of phase length control of BIODENIPHO[®] process using online ammonia and nitrate analyzers at three full-scale WWTP WEFTEC Proceedings, vol. 11, 2004, Session 11 through Session 20, pp. 215–225.
- [6] E. Ayesa, A. De la Sota, P. Grau, J.M. Sagarna, A. Salterain, J. Suescun, Supervisory control strategies for the new WWTP of Galindo-Bilbao: The long run from the conceptual design to the full-scale experimental validation, *Water Sci. Technol.* 53(4–5) (2006) 193–201.
- [7] P. Ingildsen, U. Jeppsson, G. Olsson, Dissolved oxygen controller based on on-line measurements of ammonia combining feed-forward and feedback, *Water Sci. Technol.* 45(4–5) (2002) 453–460.
- [8] G. Olsson, M. Nielsen, Z. Yuan, A. Lynggaard-Jensen, J.P. Steyer, *Instrumentation, Control and Automation in Wastewater Systems*, IWA Publishing London, UK, 2005.

- [9] B. Rusten, H. Ødegaard, Design and operation of nutrient removal plants for very low effluent Concentration, in: Proceedings of the WEF conference Nutrient Removal, Baltimore, MA, March 4–7, 2007.
- [10] H.D. Park, L.M. Whang, S.R. Reusser, D.R. Noguera, Taking advantage of aerated-anoxic operation in a full-scale university of Cape Town Process. *Water Environ. Res.* 78 (2006) 637–642.
- [11] P. Samuelsson, Control of Nitrogen Removal in Activated Sludge Processes, PhD dissertation, Uppsala University, 2005.
- [12] D. Lumley, On-line instrument confirmation: How can we check that our instruments are working? *Water Sci. Technol.* 45(4–5) (2002) 469–476.
- [13] B. Holenda, Development of Modelling, Control and Optimization Tools for the Activated Sludge Process, PhD dissertation, University of Pannonia, 2007.
- [14] A.M. Borglund, DO Control in Biological Tanks at Käppala WWTP, Internal report, Käppala WWTP, 2005.
- [15] M. Mulas, Modelling and Control of Activated Sludge Processes, PhD dissertation, University of Gagliari, (2006).
- [16] C.F. Lindberg, Control and Estimation Strategies Applied to the Activated Sludge Process, PhD Thesis, Uppsala University, 1996. Available from: <www.it.uu.se/research/syscon/automatic/modcont_waste>.
- [17] S. Capela, S. Gillot, A. Héduit, Oxygen transfer under process conditions: Comparison of measurement methods—72th Annual Conference WEFTEC'99 Municipal Wastewater Treatment Process Symposium: Aeration and oxygen transfer, New Orleans, USA, October, 1999.
- [18] P. Ingildsen, Realising Full-scale Control in Wastewater Treatment Systems, PhD dissertation, Lund University, 2002.
- [19] H.D. Park, D.R. Noguera, Characterization of two ammonia-oxidizing bacteria isolated from reactors operated with low dissolved oxygen concentrations, *J. Appl. Microbiol.* 102(5) (2007) 1401–1417.
- [20] K. Svardal, S. Lindtner, S. Winkler, Optimum aerobic volume control based on continuous in-line oxygen uptake monitoring, *Water Sci. Technol.* 47(11) (2003) 305–312.
- [21] M. Henze, C. Grady, W. Gujer, G. Marais, T. Matsuo, Activated Sludge Model No 1, Rep. 1, IWA, 1987.
- [22] U. Jeppsson, J. Alex, M.N. Pons, H. Spanjers, P.A. Vanrolleghem, Status and future trends of ICA in wastewater treatment—A European perspective, *Water Sci. Technol.* 45(4–5) (2002) 485–494.
- [23] J. Richalet, A. Rault, J.L. Testud, J. Papon, Model predictive heuristic control, *Automatica* 14 (1978) 413–428.
- [24] C.F. Lindberg, Control and Estimation Strategies Applied to the Activated Sludge Process, PhD Thesis, Uppsala University, Sweden, ISBN (1997) 91-506-1202-6.
- [25] D. Kaelin, L. Rieger, J. Eugster, K. Rottermann, C. Bänninger, H. Siegrist, Potential of in-situ sensors with ion-selective electrodes for aeration control at wastewater treatment plants, *Water Sci. Technol.* 58(3) (2008) 629–637.
- [26] D. Thauré, C. Lemoine, O. Daniel, N. Moatamri, J. Chabrol, Optimisation of aeration for activated sludge treatment with simultaneous nitrification denitrification, *Water Sci. Technol.*—WST 58(3) (2008) 639–645.
- [27] P. Regmi, M. Miller, R. Bunce, D. Hingley, D. Kinnear, B. Wett, S. Murthy, C. Bott, A Pilot Study to Evaluate the Feasibility of Mainstream Deammonification, WEFTEC Proceedings, New Orleans, 2012.
- [28] H. Littleton, G. Daigger, H. Zhao, P. Strom, D. Noguera, J. Wen, Optimization of simultaneous nitrification denitrification performance in different ENR process configurations WEFTEC proceedings session 47 through session 53 (2013) 3365–3388.
- [29] R.A. Poduska, M.K. Stenstrom, The effect of dissolved oxygen concentration on nitrification, *Water Res.* 14(6) (1980) 643–649.
- [30] D.E. Thornberg, M.K. Nielsen, K.L. Andersen, Nutrient removal: On-line measurements and control strategies, *Water Sci. Technol.* 28(11–12) (1993) 549.
- [31] D.E. Thornberg, Nitrogen removal by computer control of a simple activated sludge plant, *Water Supply* 6 (1988) 361.
- [32] T.G. Potter, B. Koopman, S.A. Svoronos, Optimization of a periodic biological process for nitrogen removal from wastewater, *Water Res.* 30(1) (1996) 142.